# R006 8 REFERENCE

**CONTENTS** 

**DESCRIPTION** 

LEGEND (SOIL & ROCK)

SUPPLEMENTAL LEGEND (GSI)

SOIL & ROCK TEST RESULTS

BORE LOGS, CORE LOGS & CORE PHOTOGRAPHS

TITLE SHEET

SITE PLAN PROFILE

CROSS SECTIONS

SHEET NO.

2A

5-8

9-19

#### STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

#### **STRUCTURE** SUBSURFACE INVESTIGATION

COUN	NTY_	JOI	HNS1	ON	7					
PROJ	JECT	DE	SCRIP	TIO	N _ <b>B</b>	RIDO	GE NO	<b>).</b> 173	<b>OVER</b>	
BU	FFA.	LO	CRE	EK	ON	SR	1700	(CO	VERED	BRIDGE
RO	AD)	BE'	TWEI	EN	SR	1003	AND	SR	3519	
SITE	DES	CRIF	PTION							

STATE PROJECT REFERENCE NO. TOTAL SHEETS BP4.R006

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABDRATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NIDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

CATLIN ENGINEERS

	AND SCIENTISTS
	·
	BY
DRAWN BY	T. LYNN
CHECKED BY _	K. BUSSEY
SUBMITTED BY	
	OBER 2022

HDR Engineering, Inc. of the Carolinas 555 Fayetteville St, Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116



SIGNATURE

**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED** 

PROJECT REFERENCE NO. SHEET NO.

BP4.R006

2

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

### SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION CONTROL TO C	ROCK DESCRIPTION  HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.  UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	AQUIFER - A WATER BEARING FORMATION OR STRATA.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.  ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED WISSON NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION		ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS	MINERALOGICAL COMPOSITION  MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAQLIN, ETC.	CRYSTALLINE CRYSTALLINE WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200)  GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-0 A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-6 A-7-6 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL 000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50	ROCK (NCR)  ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.  COASTAL PLAIN  COASTAL PLAIN  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	OF SLOPE.
7. PASSING	HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK STYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
■10 50 MX GRANULAR SIL1- MUCK,	PERCENTAGE OF MATERIAL	(CP) SHELL BEDS, ETC. WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*40 30 MX 50 MX 51 MN	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING *40	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN 11 MN MODERATE HIGHLY	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF ORGANIC SOILS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TYPES STONE FRAGS.  OF MAIDE GRAVELAND FINE SILTY OR CLAYEY SILTY CLAYEY MATTER		(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN. RATING AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
AS SUBGRADE   POOR   P	- O-MI► SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM, FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FIELD.
COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED		(MOD.SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH (N-VALUE) (TONS/FT <sup>2</sup> )	ROADWAY EMBANKMENT (RE)  25/025  DIP & DIP DIRECTION  OF ROCK STRUCTURES	IF TESTED, WOULD YIELD SPT REFUSAL  SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
GENERALLY VERY LOOSE < 4	SOIL SYMBOL SPIT DAT TEST BORING SLOPE INDICATOR INSTALLATION	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GRANULAR LUUSE 4 10 10 10 N/A	M	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.  IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS
MATERIAL DENSE 30 TO 50	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT TEST	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY DENSE         > 50           VERY SOFT         < 2	INFERRED SOIL BOUNDARY - CORE BORING SOUNDING ROD	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT 2 TO 4 0.25 TO 0.5	MW - TECT DODING	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</u>	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY   MEDIUM STIFF   4 TO 8   0.5 TO 1.0   MATERIAL   STIFF   8 TO 15   1 TO 2	INFERRED ROCK LINE MONITORING WELL WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4 HARD > 30 > 4	TTTTT ALLUVIAL SOIL BOUNDARY A PIEZOMETER ON SPT N-VALUE	ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
U.S. STD. SIEVE SIZE 4 10 40 60 200 270		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	ROCK.
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	□ UNSUITABLE WASTE □ ACCEPTABLE, BUT NOT TO BE	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - UNDERCUT UNDER	TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
(BLDR.) (COB.) (GR.) (SE. SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	MEDIUM  CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\gamma_{ m d}$ - DRY UNIT WEIGHT	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION  (ATTERBERG LIMITS) DESCRIPTION	CSE COARSE ORG ORGANIC  DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
(SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO         SD SAND, SANDY         SS - SPLIT SPOON           F - FINE         SL SILT, SILTY         ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC LIQUID LIMIT	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNALL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
RANGE - WET - (W) SEMISOLID; REQUIRES DRYING TO	FRAGS FRAGMENTS $w$ - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	BENCH MARK: N/A
(PI) PL PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE	HI HIGHLY V - VERY RATIO	TERM         SPACING         TERM         THICKNESS           VERY WIDE         MORE THAN 10 FEET         VERY THICKLY BEDDED         4 FEET	
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT  DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: FEET
SL SHRINKAGE LIMIT	CME-45C CLAY BITS X AUTOMATIC MANUAL	MODERATELY CLOSE	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	6' CONTINUOUS ELIGHT AUGER	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	BORING ELEVATIONS OBTAINED FROM SURVEY CONDUCTED BY CATLIN ENGINEERS AND SCIENTISTS
	CME-55   CORE 312E:	THINLY LAMINATED < 0.008 FEET  INDURATION	1
PLASTICITY		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	FIAD - FILLED IMMEDIATELY AFTER DRILLING
PLASTICITY INDEX (PI) DRY STRENGTH  NON PLASTIC 0-5 VERY LOW	TUNGCARBIDE INSERTS	RUBBING WITH FINGER FREES NUMEROUS GRAINS;	
SLIGHTLY PLASTIC 6-15 SLIGHT MODERATELY PLASTIC 16-25 MEDIUM	VANE SHEAR TEST CASING W/ ADVANCER HAND TOOLS:	GENILE BLOW BY HAMMER DISINIEGRATES SAMPLE.	
HIGHLY PLASTIC 26 OR MORE HIGH	POSTABLE HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	TRICONE TUNGCARB. SOUNDING ROD	GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN. RED. YELLOW-BROWN, BLUE-GRAY).	X CORE BIT VANE SHEAR TEST	DIFFICULT TO BREAK WITH HAMMER.	
		EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	X MUD ROTARY	SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14

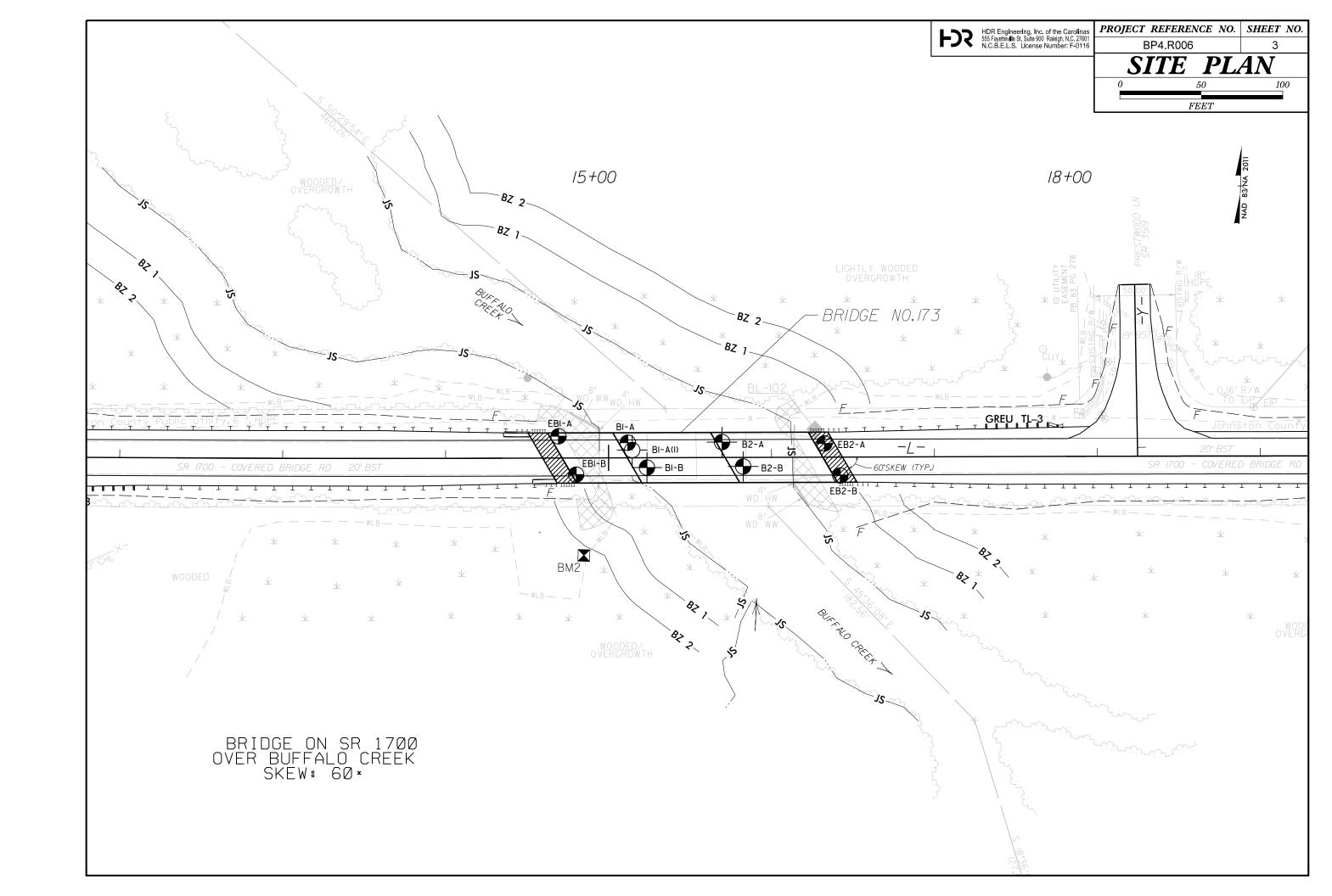
PROJECT REFERENCE NO.	SHEET NO.
BP4.R006	2A

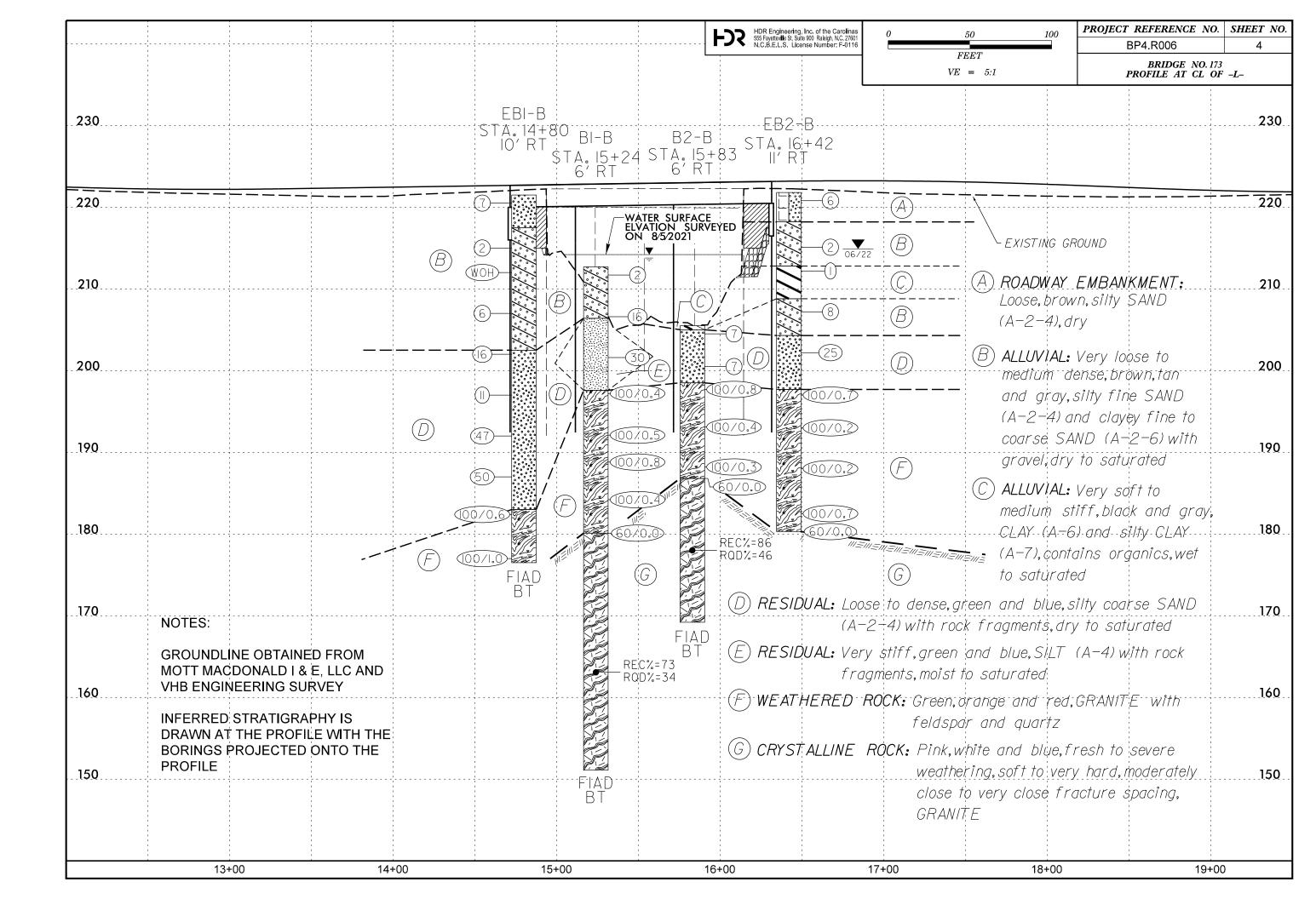
#### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

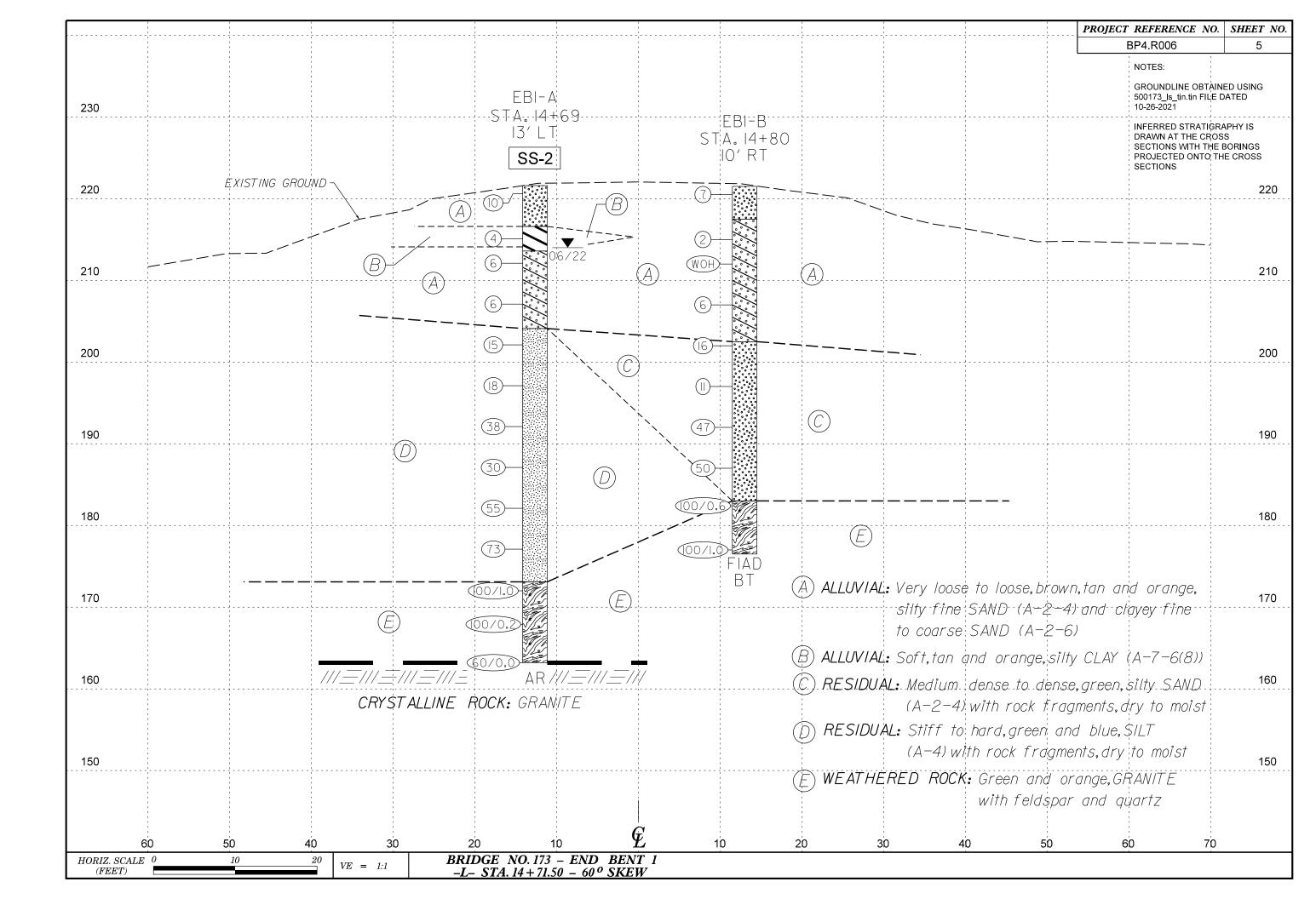
#### SUBSURFACE INVESTIGATION

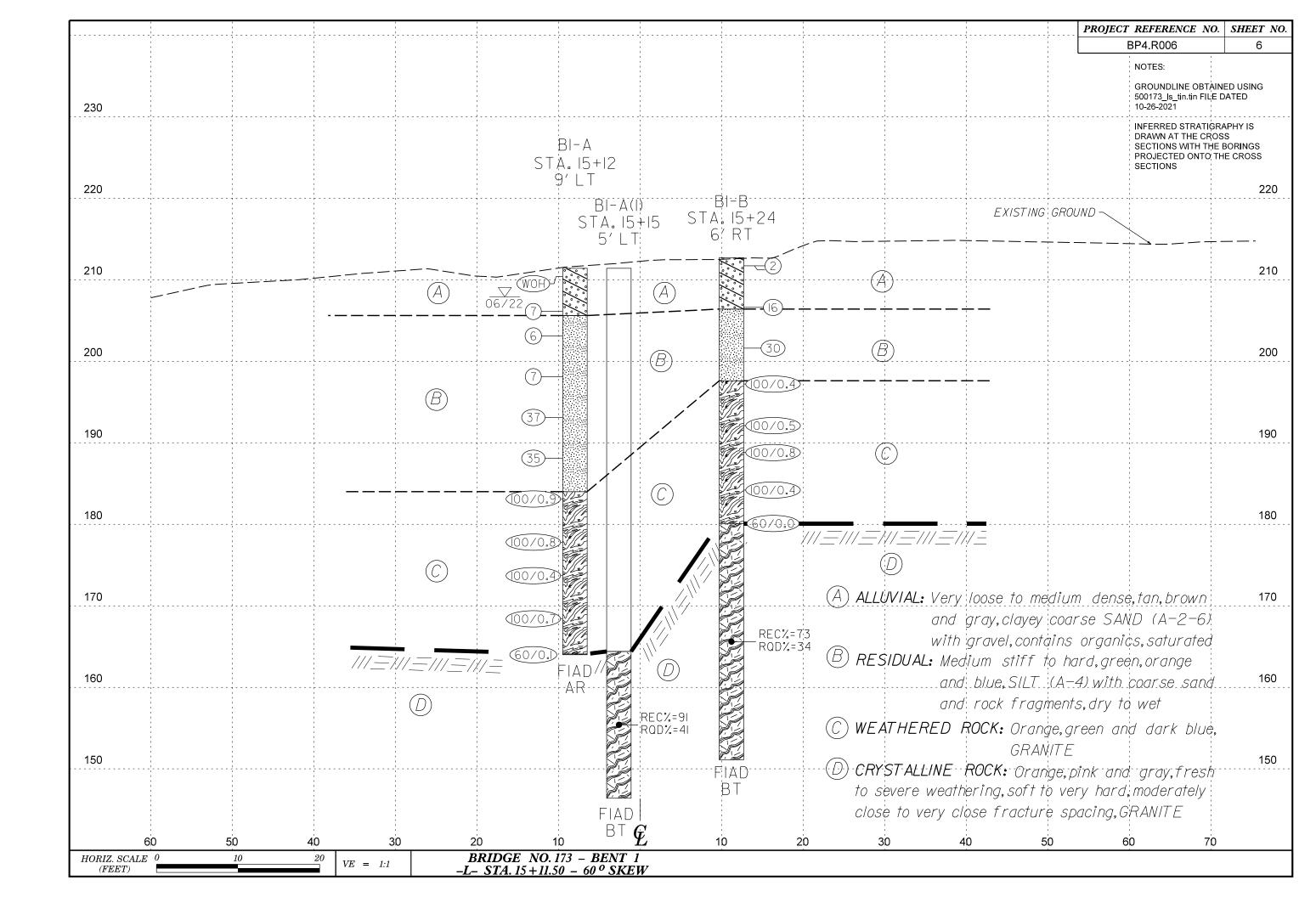
#### SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES

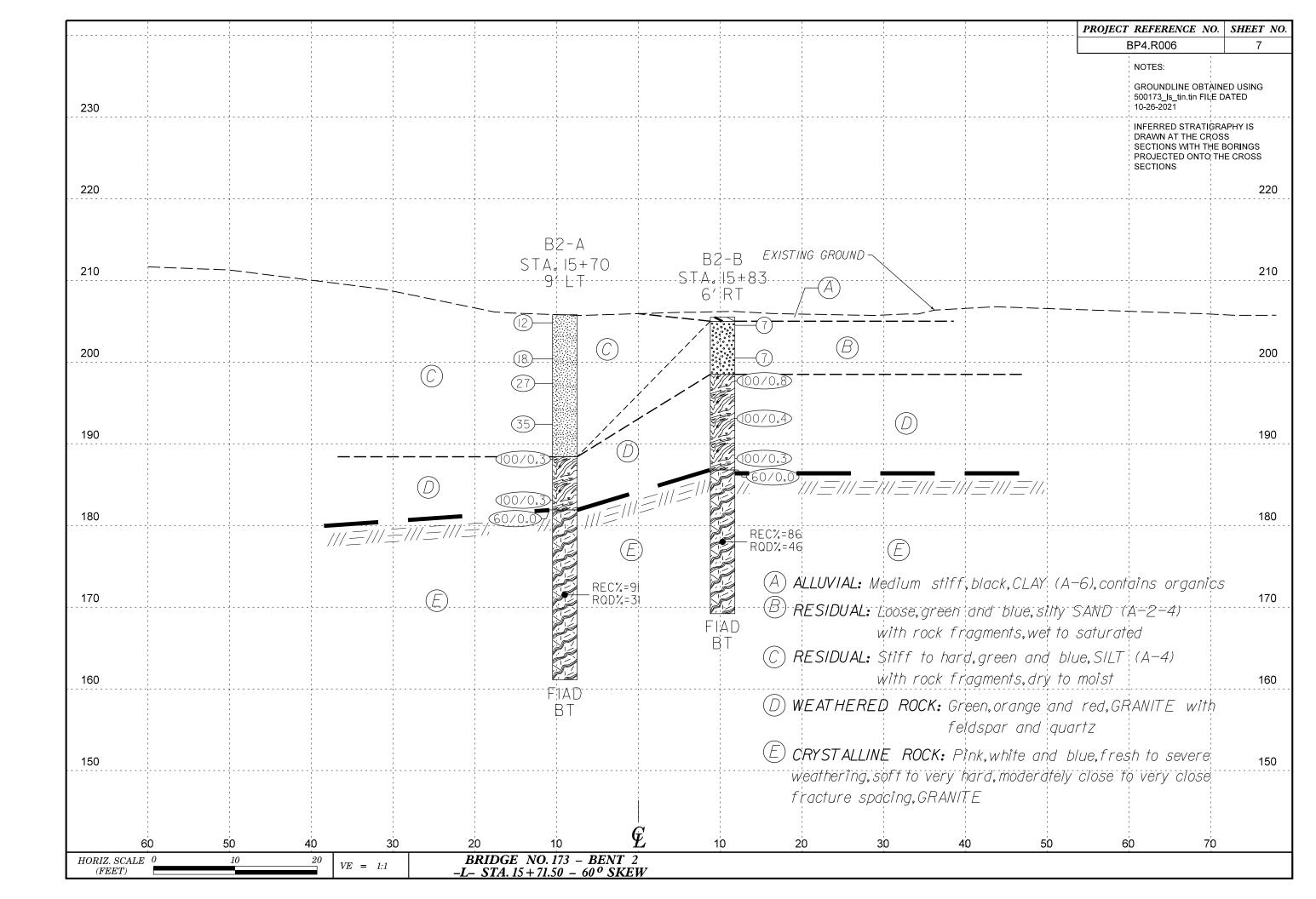
FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000) AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000) GEOLOGICAL STRENGTH INDEX (GSI) FOR GSI FOR HETEROGENEOUS ROCK MASSES SUCH JOINTED ROCKS (Hoek and Marinos, 2000) AS FLYSCH (Marinos. P and Hoek E., 2000) From a description of the lithology, structure and ,occasionally es with compact s with angular POOR - Very smooth, slicken-l or highly weathered surfaces soft clay coatings or fillings From the lithology, structure and surface and athered sur or fillings conditions of the discontinuities, estimate the average value of GSI. Do not try to surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the planes) be too precise. Quoting a range from 33 to 37 is more realistic than stating that position in the box that corresponds to the condition weathered of the discontinuities and estimate the average value GSI = 35. Note that the table does not of GSI from the contours. Do not attempt to be too apply to structurally controlled failures. Where weak planar structural planes are precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the slightly present in an unfavorable orientation smooth, c surface fillings highly coating Hoek-Brown criterion does not apply to structurally with respect to the excavation face, these will dominate the rock mass controlled failures. Where unfavourably oriented behaviour. The shear strength of surfaces continuous weak planar discontinuities are present, in rocks that are prone to deterioration slightly es these will dominate the behaviour of the rock mass. Rough, blickensided, houth compact of angular free as a result of changes in moisture content will be reduced if water is The strength of some rock masses is reduced by the 1 0 GOOD rough, presence of groundwater and this can be allowed for present. When working with rocks in the by a slight shift to the right in the columns for fair, fair to very poor categories, a shift to th, r poor and very poor conditions. Water pressure does the right may be made for wet conditions. GOOD Rough, s surface VERY | sided with s FAIR -weath VERY Slick with VERY Very VERY Water pressure is dealt with by effective FAIR Smooralter not change the value of GSI and it is dealt with by stress analysis. using effective stress analysis. STRUCTURE DECREASING SURFACE QUALITY COMPOSITION AND STRUCTURE INTACT OR MASSIVE - intact A. Thick bedded, very blocky sandstone 90 rock specimens or massive in 7Ó N/A N/A The effect of pelitic coatings on the bedding situ rock with few widely spaced planes is minimized by the confinement of PIECES discontinuities the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally 80 controlled instability. 60 BLOCKY - well interlocked un-70<sup>′</sup> disturbed rock mass consisting of cubical blocks formed by three D. Siltstone B. Sand-stone wi thin inte THIN E. Weak intersecting discontinuity sets 50 🛭 C. Sand-60 stone and stone with or silty shale siltstone /E thin inter siltstone with sandor clayey С shale with layers of an similar stone layers VERY BLOCKY - interlocked. siltstone amounts sands tone 40 partially disturbed mass with 50 multi-faceted angular blocks formed by 4 or more joint sets INTERL  $C_{\bullet}D_{\bullet}E_{\bullet}$  and G - may be more or F. Tectonically deformed, BLOCKY/DISTURBED/SEAMY -30 less folded than illustrated but intensively folded/faulted, folded with angular blocks this does not change the strength. sheared clayey shale or siltstone formed by many intersecting Tectonic deformation, faulting and with broken and deformed CREASING loss of continuity moves these discontinuity sets. Persistence sandstone layers forming an 30 categories to F and H. of bedding planes or schistosity almost chaotic structure 20 DISINTEGRATED - poorly interlocked, heavily broken rock mass 20 H. Tectonically deformed silty with mixture of angular and or clayey shale with or clayey shale forming a 10 rounded rock pieces or without a few very chaotic structure with pockets thin sandstone layers of clay. Thin layers of sandstone are transformed into small rock pieces 10 LAMINATED/SHEARED - Lack of blockiness due to close spacing N/A N/A → Means deformation after tectonic disturbance of weak schistosity or shear planes DATE: 8-19-16

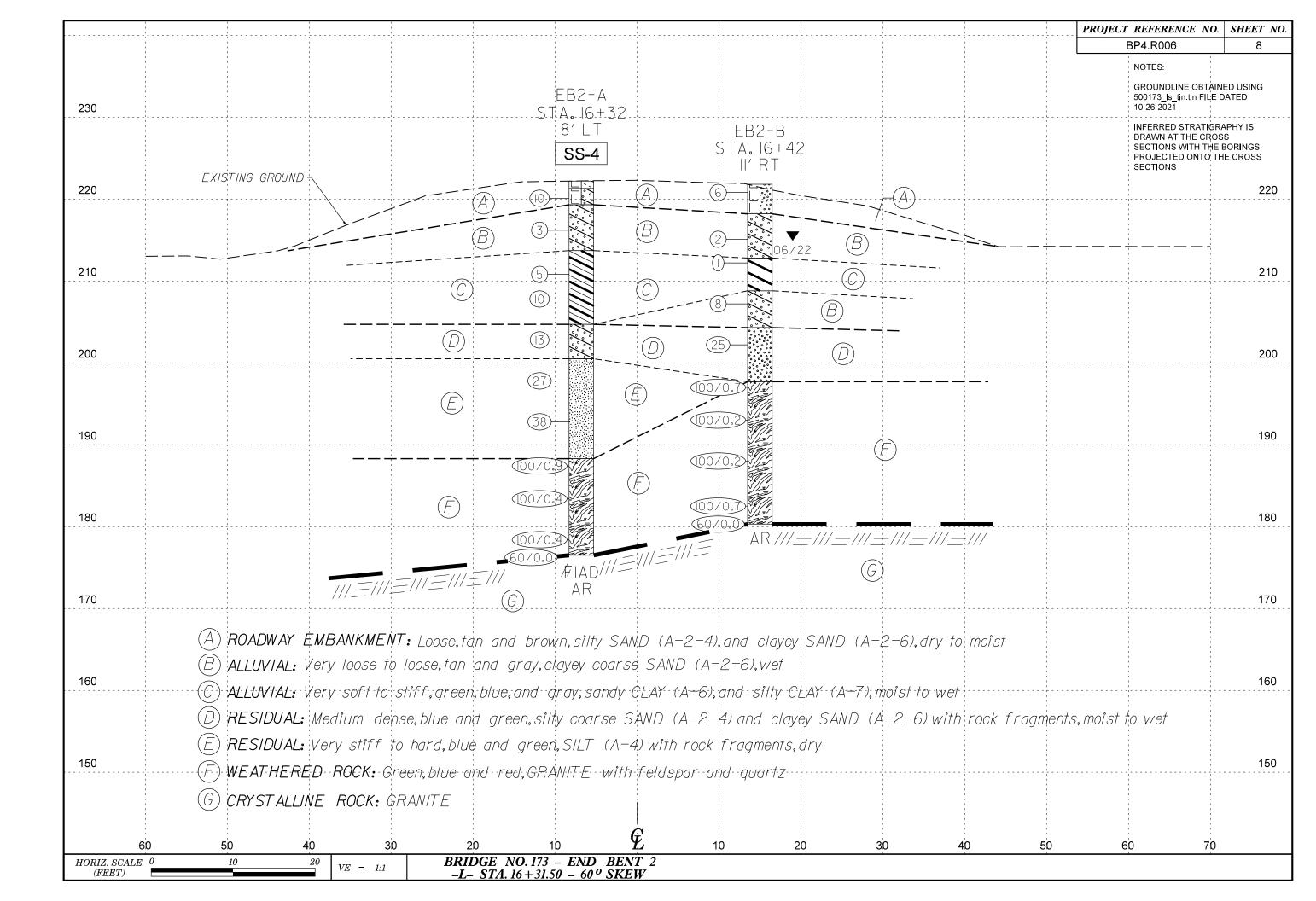


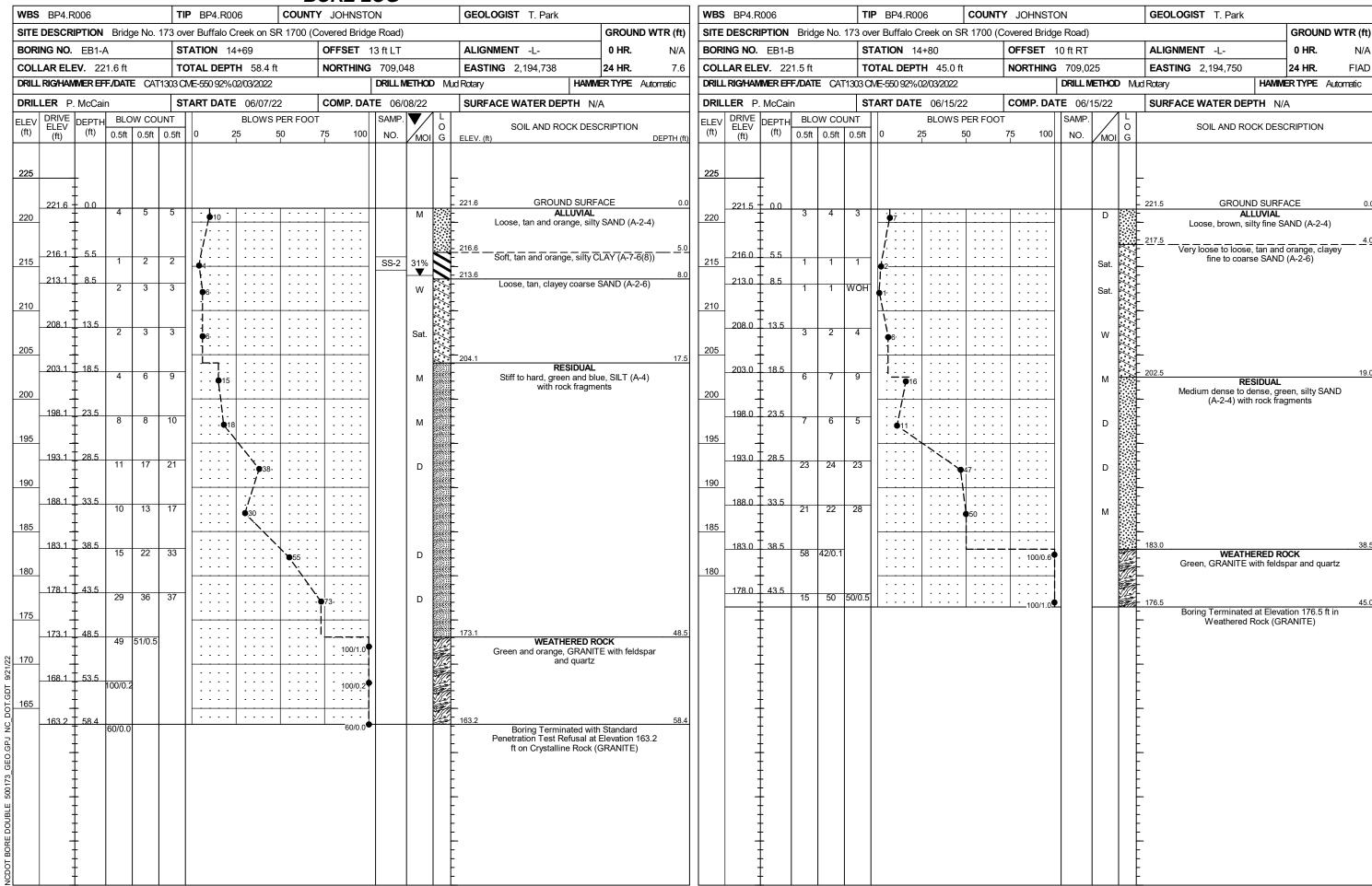












								<u> </u>	UKE L	.00				
NBS	BP4.R	2006			Т	IP BP4.R0	06	COUNT	Y JOHNST	NC			GEOLOGIST T. Park	
SITE	DESCR	IPTION	Brid	ge No.	173 o	ver Buffalo (	Creek on S	R 1700 (C	overed Bridg	e Road)				GROUND WTR (ft)
BORIN	NG NO.	B1-A			S	TATION 1	5+12		OFFSET	9 ft LT			ALIGNMENT -L-	<b>0 HR.</b> 3.5
COLL	AR ELE	<b>EV</b> . 21	1.4 ft		T	OTAL DEP	<b>TH</b> 47.4 f	t	NORTHING	709,04	15		<b>EASTING</b> 2,194,781	24 HR. FIAD
DRILL	RIG/HAN	MER EF	F./DAT	E CA	T1303 C	DME-550 92%	02/03/2022			DRILL N	ETHOD	) Muc	d Rotary HAMIN	MER TYPE Automatic
DRILL	<b>ER</b> P.	McCai	n		S	TART DAT	E 06/08/2	22	COMP. DA	TE 06/0	)8/22		SURFACE WATER DEPTH N	/A
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)		0.5ft		0	BLOWS	PER FOOT 50	75 100	SAMP.	MOI	L O G	SOIL AND ROCK DES	SCRIPTION DEPTH (f
215		_											-	
210	211.4	0.0	WOH	WOF	I WOH	<b>4</b> 0 <u>· · · · · · · · · · · · · · · · · · · </u>					Sat.		211.4 GROUND SURF ALLUVIAL Very loose to loose, tan an	
	207.1	4.3				', : : :	: : : :	: : : :			$\nabla$	$/\!\!/$	SAND (A-2-6), contain	ns organics
205	204.1	7.3	7	4	3	7					Sat.		205.6	
	-		2	3	3	- - - - - - - - - - - - - - - - - - -					W	E	Medium stiff to hard, gree sandy SILT (A-4) with ro	en and orange, ck fragments
200	199.1	12.3	3	3	4						w	F	-	
95	- - 194.1	17.3										E	-	
190	-		11	13	24		37 -				D			
90	189.1	22.3	13	15	20	<u> </u>	- 435 -				D		-	
85	184.0	27.4		45/0									184.0	27
180	-	-	55	45/0.4	1				100/0.9				<b>WEATHERED R</b> Orange and green, 0	
	179.1	32.3	19	49	51/0.3	3			100/0.8	•			-	
75	174.1 _	37.3	100/0.4	4					100/0.4	,			-	
70	-	40.0											-	
	169.1 <u> </u>	42.3	39	61/0.2	2				- 100/0.7					
165	164.1												164.1	47
			60/0.1						60/0.1				CRYSTALLINE F Green, GRAN Boring Terminated wit Penetration Test Refusal at ft in Crystalline Rock (	ROCK TE h Standard Elevation 164.0

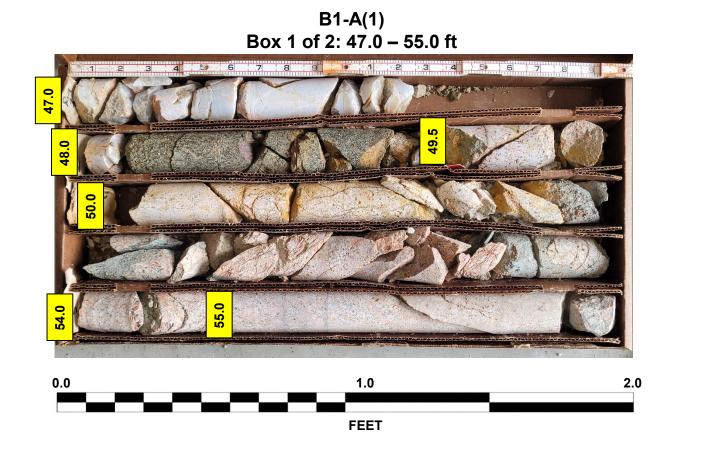
SHEET 10

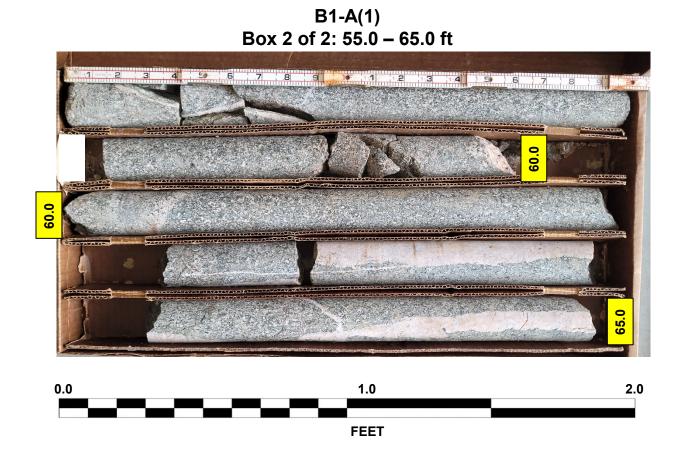
										<u> </u>				T	
	BP4.R					P BP4.R		 COUNT						GEOLOGIST T. Park	Т
				ge No.		er Buffalo		R 1700 (C						1	GROUND WTR (ft)
	G NO.					TATION				SET 5				ALIGNMENT -L-	<b>0 HR</b> . N/A
	AR ELE					OTAL DEF			NOR	THING	709,04			<b>EASTING</b> 2,194,784	24 HR. FIAD
RILL R	RIG/HAM	MER EF	F./DAT	E CA		ME-550 92%					DRILL IV	<b>IETHOD</b>	Muc	d Rotary HAMIN	MERTYPE Automatic
	ER P.	McCai				TART DAT			ļ	P. DA1	<b>FE</b> 06/0	08/22		SURFACE WATER DEPTH N/	'A
.EV   E	ELEV	DEPTH (ft)		W CO				PER FOOT		100	SAMP.		0	SOIL AND ROCK DES	CRIPTION
+	(ft)	(11)	0.5ft	0.5ft	0.5ft	0	25	 60 I	75	100	NO.	MOI	G	ELEV. (ft)	DEPTH (f
15		-											Ŀ	-	
	1	-											E	211.4 GROUND SURF	ACE 0.
10	1	-						 		• •			ŧ	See bore log for B1-A for se	
	1	-					.   .	 					E		
05	‡	-					:   : :	 	: :				E		
05	+	-					+	 	+				-	-	
	‡	<u>-</u>					:   : :	 	: :				ļ		
00	‡	-					<u> </u>	 	<u> </u>				E	-	
	‡	-					:   : :	 	: :				þ		
95	‡	<del>-</del> -					·   - ·	 		: :			ļ		
,5	1	-						 	1				F	-	
	‡	-					.	 	: :				F		
90	‡	-						 	1:-				F	-	
	‡	-					:   : :	 	: :				F		
35	1	- -					.	 					F		
	7	-						 					F	•	
	7	-					.   : :	 	: :				F		
30		-							+:				E	-	
	$\exists$	-											E		
75	1	-						 	<u> </u>				Ŀ		
	1	-					.	 					E		
70	1	<u>-</u> -					.   <u>.</u> .	 					Ŀ		
70	+	-					-	 	+ : :				-	-	
	‡	- -					.	 					ţ		
35	‡	-						 	<u> </u>				Ė	<sup>-</sup> 164.4	47.
	‡	-					.   : :	 	: :					CRYSTALLINE R Pink, blue, and dark gray, (	ROCK GRANITE with
60	‡	<u>-</u>					.	 	: :					feldspar and qu	artz
50	7	-						 	1					-	
	‡	- -					.   : :	 	: :						
55	- ‡	-						 	+						
	†	- -													
50	†	- -													
7	$\exists$	-						 		: :					
	}	<b>-</b>					.   - :	 	1		1		1	146.4 Poring Terminated at Flour	65.
	-	-											E	Boring Terminated at Eleva Crystalline Rock (GF	RANITE)
	]	- -											E		
	E	-											E		
	$\exists$	<b>-</b>											E		
	7	-											F		

# GEOTECHNICAL BORING REPORT CORE LOG

WBS	BP4.R0	006			TIP	BP4.F	2006	C	TNUC	<b>Y</b> J	JOHNSTON GEOLOGIST T. Park
SITE	DESCRI	PTION	Bridg	ge No. 17	3 over	Buffalo	Creek or	SR 1	700 (C	Cove	vered Bridge Road) GROUND WTR
BORI	NG NO.	B1-A(	(1)		STAT	ΓΙΟN	15+15			OF	DFFSET 5 ft LT ALIGNMENT -L- 0 HR.
COLI	LAR ELE	<b>V</b> . 21	1.4 ft		TOT	AL DE	<b>PTH</b> 65.	0 ft		NC	NORTHING 709,041 EASTING 2,194,784 24 HR. FI.
DRILL	.RIG/HAMI	MER EF	F./DATI	E CAT13	33 CME	-550 92	%02/03/20	22			DRILL METHOD Mud Rotary HAMMER TYPE Automatic
DRIL	LER P.	McCai	n		STA	RT DA	<b>TE</b> 06/0	8/22		CC	COMP. DATE 06/08/22 SURFACE WATER DEPTH N/A
COR	E SIZE	NQ2			TOTA	AL RUI	<b>1</b> 18.0 ft				
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft) %	JN RQD (ft) %	SAMP. NO.	STR REC. (ft) %	RQD (ft) %	L O G	D DESCRIPTION AND REMARKS
164.4	184:4 1	47.0	1.0	2:30	(1.0)	(0.0)		(16.4)	(7.0)		Begin Coring @ 47.0 ft CRYSTALLINE ROCK
160 155 150	157.4 156.4 151.4	49.5 50.0 50.0 55.0 55.0 60.0	1.5 0.5 4.0 1.0 5.0	1:36 1:20/0.5 0:43/0.5 0:56 1:09 0:35 1:29 2:15 1:36 1:21 1:17 1:19 1:20 1:22 1:20 1:28 1:39	(0.5) (0.5) (0.6) (0.5) (0.5) (0.5) (0.5) (0.5) (0.5) (0.5) (0.5) (0.5) (0.5)	0% (0.0) 0% (0.0) 0% (0.0) 0% (2.3) 46% (5.0) 100%		91%	(7.3) 41%		Pink, blue, and dark gray, fresh to severe weathering, soft to very hard, moderately close to very close fracture spacing, GRANITE with feldspar and quartz  GSI = 30-40
	146.4	- 65.0 - - - -		1:40							Boring Terminated at Elevation 146.4 ft in Crystalline Rock (GRANITE)
	+	- - - - - - -									- - - - - - - -
	+	- - - - - -									-
	+	- - - - - -									-
	-	- - - - - -									- - - - - - - -
		• • • •									
	‡ + +	- - - - -									

Bridge No. 173 over Buffalo Creek on SR 1700 (Covered Bridge Road)



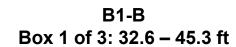


							B	<u>ORE L</u>	<u>OG</u>				
WBS	BP4.R0	006			TII	<b>P</b> BP4.R006	COUNTY	/ JOHNSTO	N			GEOLOGIST T. Park	
SITE	DESCRIF	PTION	Brido	ge No.	173 ov	er Buffalo Creek on S	R 1700 (C	overed Bridge	e Road)				GROUND WTR (ft)
BORI	ING NO.	В1-В			SI	<b>TATION</b> 15+24		OFFSET 6	ft RT			ALIGNMENT -L-	<b>0 HR</b> . N/A
COLI	LAR ELE	<b>V.</b> 21	2.7 ft		т	OTAL DEPTH 61.6 f	t	NORTHING	709,03	30		<b>EASTING</b> 2,194,793	24 HR. FIAD
DRILL	RIG/HAMI	VIER EF	F./DAT	E CAT	1303 C	ME-550 92%02/03/2022			DRILL M	ETHOD	Muc	d Rotary HAMM	ER TYPE Automatic
DRIL	LER P.	McCai	n		ST	TART DATE 06/10/2	2	COMP. DAT	Γ <b>E</b> 06/1	4/22		SURFACE WATER DEPTH N/	Α
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLC 0.5ft	0.5ft	JNT 0.5ft	BLOWS 0 25	PER FOOT	75 100	SAMP. NO.	<b>"/</b>	L O G	SOIL AND ROCK DES	CRIPTION DEPTH (ft)
215	040.7										F	- 212.7 GROUND SURF	ACE 0.0
210	212.7	0.0	WOH	1	1	2				Sat.		ALLUVIAL  Very loose to medium dense- clayey coarse SAND (A-2-	
	207.6	5.1				:\:: ::::				% % o%	//	, ,	, 0
205	+		5	6	10	16				W		RESIDUAL  Very stiff, green and blue, S	
	202.6	10.1	12	15	15	• • • • • • • • • • • • • • • • • • •				М	F	rock fragment	is .
200	197.6	15.1						<u> </u>				197.6	15.1
195	†	·	100/0.4					100/0.4				<b>WEATHERED R</b> ( Dark blue, GRAN	
	192.6	20.1	100/0.5					100/0.5					
190	189.6	23.1	16	84/0.3				100/0.8				-	
185	184.6	28.1	100/0				: : : :			\$5) \$5)		-	
			100/0.4					100/0.4					
180	180.1	32.6	60/0.0					60/0.0				CRYSTALLINE R Pink, tan, and gray, G	
175	<del> </del>	• •										-	
170		•										-	
165													
160		•							RS-1			-	
155		•										-	
	<u> </u>											151.1	61.6
											<u>-</u>	Boring Terminated at Eleva Crystalline Rock (GF	ation 151.1 ft in RANITE)
											Ė	-	
											Ē		
		•									-	-	
	<u> </u>										-		

#### GEOTECHNICAL BORING REPORT CORE LOG

									C	Ui	KE L	UG						
WBS	BP4.R0	006			TIP	BP4.F	R006	C	OUNT	<b>Y</b> J	OHNSTO	N		GEOLOG	SIST T. Pa	ark		
SITE	DESCRIF	PTION	Brido	ge No. 17	3 over	Buffalo	Creek o	n SR 1	700 (C	Cover	red Bridge	e Road)					GROU	ND WTR (ft)
BORI	NG NO.	В1-В			STA	TION	15+24			OF	FSET 6	ft RT		ALIGNM	ENT -L-		0 HR.	N/A
COLI	AR ELE	<b>V</b> . 21	2.7 ft		тот	AL DE	<b>PTH</b> 61.	.6 ft		NO	RTHING	709,030		EASTING	2,194,79	93	24 HR.	FIAD
DRILL	.RIG/HAMI	VIER EF	F./DAT	E CAT130	3 CME	-550 92	%02/03/20	22				DRILL MET	HOD Mud	Rotary		HAN	MER TYPE	Automatic
DRIL	LER P. I	McCai	n		STAI	RT DA	<b>TE</b> 06/1	0/22		СО	MP. DAT	<b>E</b> 06/14/	22	SURFAC	E WATER	DEPTH I	N/A	
CORI	E SIZE	NQ2			TOTA	AL RUI	<b>N</b> 29.0 f	t										
ELEV	RUN ELEV	DEPTH	RUN	DRILL RATE	REC.	JN RQD	SAMP.	STR REC.	ATA RQD	Г			Di	=SCRIPTIC	N AND REI	IARKS		
(ft)	(ft)	(ft)	(ft)	(Min/ft)	REC. (ft)	(ft) %	NO.	(ft) %	RQD (ft) %	Ğ	ELEV. (f	:)			IN AND INC.	IAITIO		DEPTH (ft
18801	100 1	20.6						(0.4.0)	(0.0)						ring @ 32.			
	180.1 T	32.6	5.0	0:31 0:43	(3.0)	(0.0) 0%		(21.3) 73%	(9.8) 34%		180.1			ay, fresh to	ALLINE RO	nering, soft		
475	475.4	27.0		1:25 1:23								moderate	ly close to v		acture spac nd quartz	ing, GRAN	TE with feld	spar
175	175.1	37.6	2.8	1:53 1:28	(0.6)	(0.0)					_			G	SI = 30-40			
	172.3	40.4		1:30 3:00/0.8	21%	0%					E							
170	170.1	42.6	2.2	1:54/1.2 3:03	(0.5) 23%	(0.0) 0%					L							
	I		3.7	2:23 2:10	(1.8) 49%	(0.0) 0%					E							
		46.3	4.2	2:23 2:00/0.7	(0.4)	(0.0)					_							
165	165.1	47.6	1.3 5.0	2:50/1.3 3:53	(0.4) 31%	(0.0) \_0%					_							
	Ŧ			1:24 1:45	(5.0) 100%	(4.0) 80%					F							
160	160.1 T	52.6		2:18 2:21			RS-1	-			F			Qu	= 5,460 psi			
	Ŧ		5.0	2:46 3:27	(5.0) 100%	(3.9) 78%	1.01	1			-			Qu	о, тоо рог			
	Ŧ			3:47 4:07							F							
155	155.1	57.6	4.0	4:33 4:50	(5.0)	(1.9)					F							
	‡			6:05 4:16	125%						-							
	151.1	61.6		5:36							151.1	Boring To	erminated a	nt Flevation	151.1 ft in C	rvstalline R	lock (GRAN	61.6
	1	•									-	9				.,	(	
	‡										-							
	‡										_							
	‡										-							
	‡										_							
	+										_							
	‡										_							
											_							
	‡										-							
	‡										_							
	+	•									_							
	‡										_							
	土										_							
	1										_							
	$\pm$										_							
	Ŧ	•									_							
	Ŧ										-							
	‡										F							
	†	•									-							
	‡										<u> </u>							
	‡										_							
	1										<u> </u>							
	$\frac{1}{2}$										_							
	1										F							
	‡																	
	1										F							

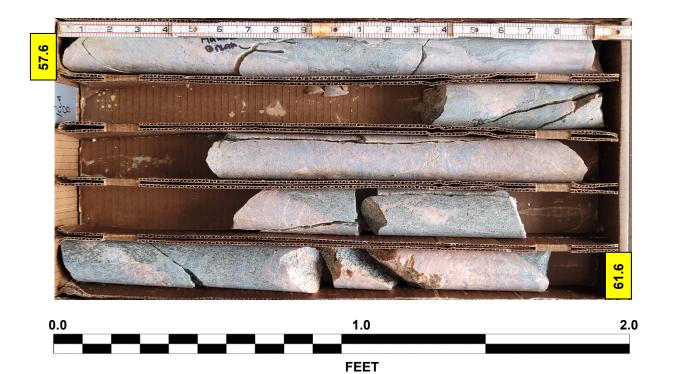
Bridge No. 173 over Buffalo Creek on SR 1700 (Covered Bridge Road)



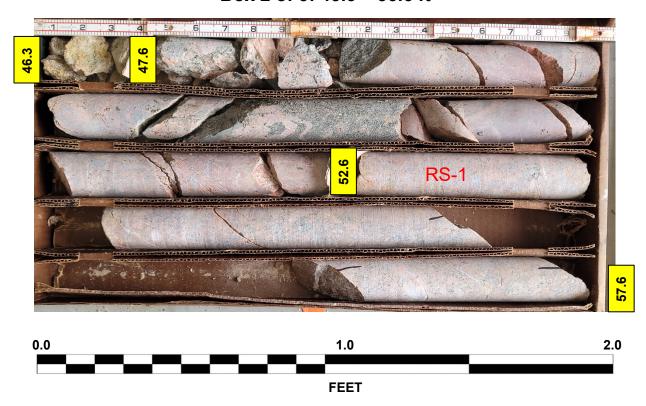


B1-B Box 3 of 3: 56.6 – 61.6 ft

FEET



B1-B Box 2 of 3: 45.3 – 56.6 ft



GEOTECHNICAL BORING REPORT

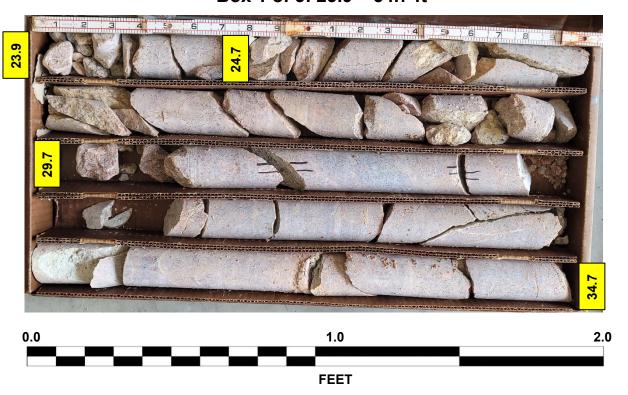
# GEOTECHNICAL BORING REPORT

# **BORE LOG**

	GEOTECHN	BORE LOG	EPORI						'	GEUT	ЕСП		ORE LO	RING RI OG	EPORT	
<b>WBS</b> BP4.R006	TIP BP4.R006 COL	UNTY JOHNSTON	GEOLOGIST T. Park		WBS	BP4.R00	)6		TIP B	P4.R006	C	YTNUC	JOHNSTON		GEOLOGIST T. Park	
SITE DESCRIPTION Bridge No. 1	73 over Buffalo Creek on SR 170	00 (Covered Bridge Road)		GROUND WTR (ft)	SITE	DESCRIP	<b>FION</b> B	ridge No. 1	73 over Bu	uffalo Creek	on SR 17	700 (Co	vered Bridge F	Road)		GROUND WTR (ft)
BORING NO. B2-A	STATION 15+70	OFFSET 9 ft LT	ALIGNMENT -L-	<b>0 HR</b> . N/A	BOR	RING NO.	32-A		STATI	<b>ON</b> 15+70			OFFSET 9 ft	LT	ALIGNMENT -L-	<b>0 HR</b> . N/A
COLLAR ELEV. 205.8 ft	TOTAL DEPTH 44.7 ft	NORTHING 709,047	<b>EASTING</b> 2,194,839	24 HR. FIAD		LAR ELEV				_ DEPTH 4			NORTHING 7		<b>EASTING</b> 2,194,839	24 HR. FIAD
DRILL RIG/HAMMER EFF/DATE CAT1		DRILL METHOD		MIMIER TYPE Automatic				DATE CAT1		50 92%02/03/				RILL METHOD		HAMMER TYPE Automatic
DRILLER P. McCain	START DATE 06/07/22	COMP. DATE 06/07/22	SURFACE WATER DEPTH	N/A		LER P. N				DATE 06		- 1	COMP. DATE	06/07/22	SURFACE WATER DEP	TH N/A
ELEV DRIVE DEPTH BLOW COU		75 400	SOIL AND ROCK DI		-	RE SIZE N		I DBILL		RUN 20.8		ΔΤΔ	. 1			
(II) (ft) (II) 0.5ft 0.5ft	0.01 0 20 00	75 100 NO. MOI	G ELEV. (ft)	DEPTH (ft)	ELEV (ft)	RUN ELEV (ft)	EPTH RI (ft) (1	JN DRILL RATE (Min/ft	REC. I	RQD SAMF (ft) NO.	STR REC. (ft) %	RQD (ft)	0		DESCRIPTION AND REMARK	
210					181.9	(10)		(IVIIII/IC	.)   %	%	%	%	G ELEV. (ft)		Begin Coring @ 23.9 ft	DEPTH (f
+			-		180	181.9	23.9 0	.8 2:15/0.	8 (0.8) (	(0.0)	(19.0)	(6.4)	_ 181.9	Pink white a	CRYSTALLINE ROCK and blue, fresh to severe weathering	23.
205.8 + 0.0			E 205.8 GROUND SUI	RFACE 0.0		1 ‡	5	1:24 1:38 3:01 4:07	(3.2) (64%	(0.0)	9170	3170		moderately close	e to very close fracture spacing, C and quartz	GRANITE with feldspar
	6		RESIDUA - Stiff to hard, green and	AL		176.1	29.7	2:14							GSI = 30-40	
201.4 T 4.4			with rock fragment	ts, saprolitic	175	<del> </del>	5	.0 2:26 2:32	(5.0) ( 100% 2	(1.3) 25%			161.1		301 - 30 40	
200 + 4.4 5 8	10	м	_			171.1	84.7	2:51 5:50 3:30								
198.4 + 7.4   10   12	15	· · ·   · · · ·       D	- -		170			.0 3:33 2:43	(5.0) ( 100% <sup>2</sup>	(2.3)						
195						‡		3:29 2:38	10070	RS-2	$\exists   $				Qu = 4,840 psi	
193 4 + 12 4			-		165	166.1	39.7	3:18 .0 3:36 2:46	(5.0)	(2.8)						
6 12 I	- $\P^{35}$ -	D D				1 I		3:39	100%	56%						
190				17.4		161.1	14.7	3:19 3:44								44.
188.4 † 17.4   	. \( \dots - \dots - \dots \)	100/0.3		ROCK									-	Boring Terminal	ted at Elevation 161.1 ft in Crysta	line Rock (GRANITE)
185			Dark green, GRANITE veguartz  181.9  CRYSTALLINE Pink, white, and blue			‡							-			
183.4		· · ·   · · · ·	181.9	23.9		‡							<u> </u>			
180 23.9 60/0.0		- · ·   · 60/0.0 •	CRYSTALLINE Pink, white, and blu	ROCK		‡							F			
<del>                                      </del>			Filik, Wilite, and blu	le, GRANITE		‡							F			
						l Ŧ							E			
175		<del>  </del>				±							L			
						‡							-			
170						‡							-			
		RS-2				‡							F			
165													Ē			
						l I							E			
			- A 101.1	44.7		‡							-			
			Boring Terminated at Ele Crystalline Rock (	evation 161.1 π in (GRANITE)		‡							<u> </u>			
			-			‡							F			
+			-		5/22	‡							F			
1 9/2			-		10/ 0/	‡							F			
			_		7T.GD								[			
					) D	‡							_			
			-		Z Z	‡							-			
9.0			-		EO.G	‡							-			
173 0			-		173_G	‡							-			
2005			-		200	‡							F			
ONBEE			E		UBLE								E			
			Ł		E DO											
			‡		, CO,	‡							<u> </u>			
			‡		CDO	‡							ļ.			
z <u>_</u>					z											

Bridge No. 173 over Buffalo Creek on SR 1700 (Covered Bridge Road)

B2-A Box 1 of 3: 23.9 – 34.7 ft



B2-A Box 3 of 3: 39.7 – 44.7 ft





B2-A Box 2 of 3: 34.7 – 39.7 ft

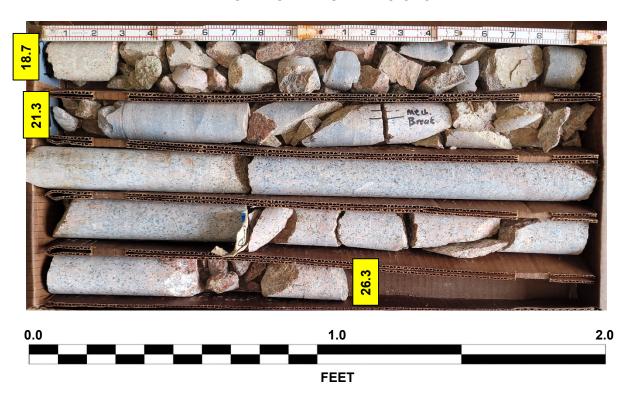


#### GEOTECHNICAL BORING REPORT CORE LOG

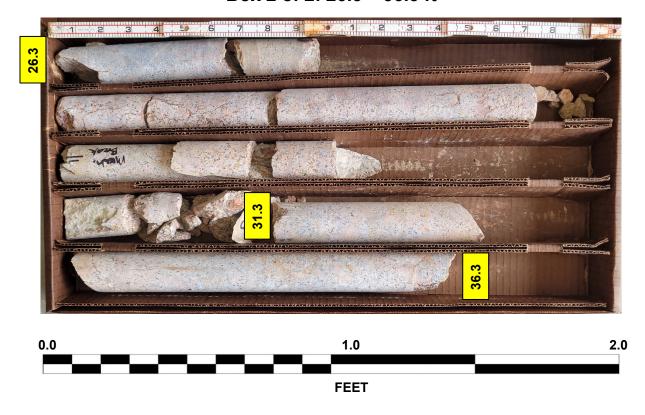
		BOI	RE LOG										CORE LOG		
WBS BP4.R006	TIP BP4.R006	COUNTY J			GEOLOGIST T. Park		WE	<b>BS</b> BP4.R006		TIP BP4.I	R006		TY JOHNSTON	GEOLOGIST T. Park	
SITE DESCRIPTION Bridge No	. 173 over Buffalo Creek or	SR 1700 (Cove	ered Bridge Road)		•	GROUND WTR (ft)	SIT	TE DESCRIPTION Brid	dge No. 17	73 over Buffal	lo Creek or	SR 1700	(Covered Bridge Road)	•	GROUND WTR (f
BORING NO. B2-B	STATION 15+83	OF	FSET 6 ft RT		ALIGNMENT -L-	0 HR. N/A	во	PRING NO. B2-B		STATION	15+83		OFFSET 6 ft RT	ALIGNMENT -L-	0 HR. N/
COLLAR ELEV. 205.5 ft	TOTAL DEPTH 36.3	3 ft NC	<b>DRTHING</b> 709,0	33	<b>EASTING</b> 2,194,852	<b>24 HR.</b> FIAD	СО	DLLAR ELEV. 205.5 ft	t .	TOTAL DE	<b>EPTH</b> 36.3	3 ft	<b>NORTHING</b> 709,033	<b>EASTING</b> 2,194,852	<b>24 HR</b> . FIA
DRILL RIG/HAMMER EFF/DATE CA	AT1303 CME-550 92% 02/03/202	2	DRILL	<b>METHOD</b> Mud	Rotary <b>HAMM</b>	MER TYPE Automatic	DRI	ILL RIG/HAMMER EFF./DA	TE CAT13	303 CME-550 92	2%02/03/202	2	DRILL METHOD	Mud Rotary 1	HAMMER TYPE Automatic
DRILLER P. McCain	START DATE 06/15	5/22 <b>CC</b>	OMP. DATE 06/	15/22	SURFACE WATER DEPTH N/	/A	DR	RILLER P. McCain		START DA	ATE 06/1	5/22	COMP. DATE 06/15/22	SURFACE WATER DEPTI	H N/A
ELEV DRIVE DEPTH BLOW CO	DUNT BLOW	'S PER FOOT	SAMP.	V L C	SOIL AND ROCK DES	COUDTION	СО	DRE SIZE NQ2		TOTAL RU	JN 17.6 ft			· · · · · · · · · · · · · · · · · · ·	
ELEV CHI DEPTH BLOW CO (ft) (ft) 0.5ft 0.5ft	t 0.5ft 0 25	50 75	100 NO.	1/1	ELEV. (ft)	DEPTH (ft)	ELE (ft)	RUN DEPTH RUN (ft)	DRILL RATE (Min/ft)	RUN REC. RQD (ft) (ft) %	SAMP. NO.	STRATA REC. RQI (ft) (ft) %	D C C ELEV. (ft)	DESCRIPTION AND REMARKS	DEPTI
210							186.	8						Begin Coring @ 18.7 ft	52
							185	1868   187   26	0:37/0.6 2:37 4:49 2:24	(1.9) (0.0) 73% 0% (4.7) (2.8)		(15.2) (8.1 86% 46%	Pink, white, a moderately clos	CRYSTALLINE ROCK and blue, fresh to severe weathering se to very close fracture spacing, GF	1 , soft to very hard, RANITE with feldspar
205 205.5 + 0.0 3 4	3 - 1			Sat.	205.5 GROUND SURF	ACE 0.0			2:24 2:24 2:10	94% 56%				and quartz	
				Joan Joan	Medium stiff, black, CLAY organics	(A-6), contains	180	0 179.2 26.3	1:58 1:49 1:56					GSI = 30-40	
201.5 + 4.0	<u> </u>			w	RESIDUAL			5.0	1:56 3:01	(4.2) (2.9) 84% 58%					
198.5 + 7.0	<u> </u>			∷∷ <b>:</b> ⊢	Loose, green, and blue, silty with rock fragme	y SAND (A-2-4) ents	175	_	2:20	0470   3070					
198.5 7.0 49 51/0.			100/0.8		WEATHERED R	OCK	175	1/4.2 31.3	2:20 2:32 2:09 2:44	(4.4) (2.4)	4				
195					Green, orange, and red, G feldspar	SKAINITE WILLI		5.0	2:18	(4.4) (2.4) 88% 48%					
193.5 + 12.0   100/0.4							170	0 169.2 36.3	1:45 2:10 1:38						
T 100/0.4		-     -	100/0.4					109.2   30.3	1:38		1		Boring Termina	ated at Elevation 169.2 ft in Crystallin	ne Rock (GRANITE)
190								1 1					1 -		
188.5 + 17.0 186.8 + 18.7			100/0.3		186.8	18.7							F		
186.8 + 18.7   60/0.0			60/0.0		CRYSTALLINE R	ROCK							F		
+					Pink, white, and blue,	GRANITE									
180 ±															
$\overline{}$								1 ± 1					<u> </u>		
		-     -						1 1							
175															
								‡					F		
1 1 1			: : : :												
170						36.3									
				[	Boring Terminated at Eleva Crystalline Rock (GF	RANITE)							-		
				1 I <u>E</u>											
				l I F				1 ± 1					1 <u>E</u>		
								+					-		
													F		
								‡							
<u> </u>							21/22								
							/6 L						-		
<del> </del>							T.GD	1 1					<u> </u>		
							00								
							N						F		
							J.GP.								
				[			GEC								
							173								
				F			200	1 1					1 - E		
				F			JBLE	+					-		
				‡			DOI						F		
,							ORE								
							ОТС	‡					-		
.							Ö	+					-		

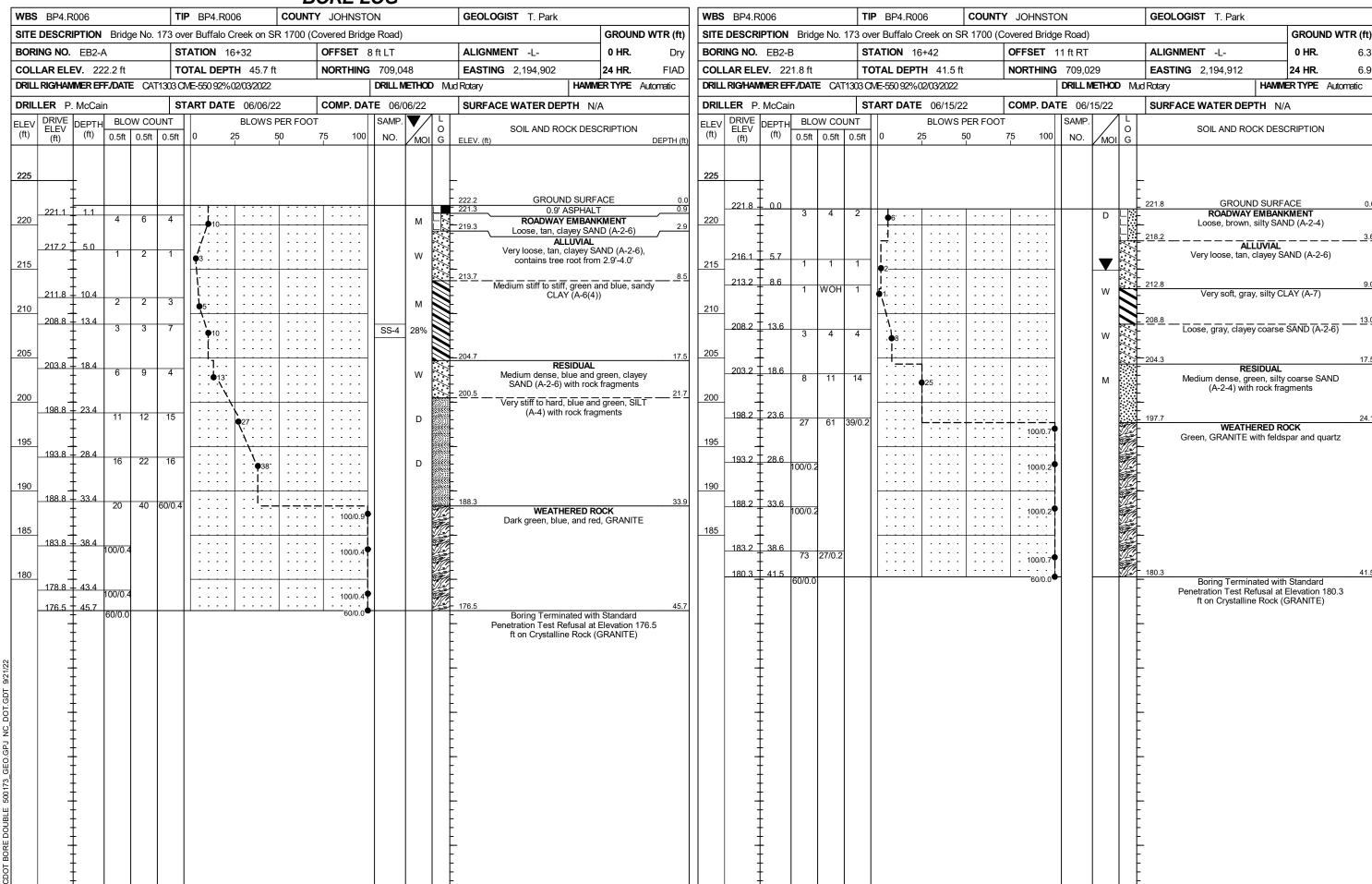
Bridge No. 173 over Buffalo Creek on SR 1700 (Covered Bridge Road)

B2-B Box 1 of 2: 18.7 – 26.3 ft



B2-B Box 2 of 2: 26.3 – 36.3 ft





												PROJE	CT NO.	BP4	4.R006	SHE	ET 20
						SOIL	TEST	RES	SULTS							_	
SAMPLE	DODINO	OT A TION	055057		DEPTH	4.4.0U.T.O. OL 4.00							% F	PASSING S	SIEVES	0/ 14 : /	%
NO.	BORING	STATION	OFFSET	LINE	INTERVAL	AASHTO CLASS.	L.L.	P.I.	C. SAND	F. SAND	SILT	CLAY	10	40	200	% Moisture	ORGANIC
SS-2	EB1-A	14+69	13' LT	-L-	5.5-7.0	A-7-6 (8)	44	19	23.9	23.0	22.0	31.2	96.2	82.0	53.6	30.8	-
SS-4	EB2-A	16+32	8' LT	-L-	13.4-14.9	A-6 (4)	31	15	10.5	30.8	22.5	36.2	99.8	95.9	63.6	28.0	-

	LABORATORY SUMMARY FOR ROCK CORE														
SAMPLE NO.	BORING	STATION	OFFSET	LINE	DEPTH INTERVAL	ROCK TYPE	GEOLOGIC MAP UNIT	RUN REC	RUN RQD	RUN LENGTH (FT)	DIAMETE R (IN)	UNIT WEIGH T (PCF)	UNCONFINED COMPRESSIVE STRENGTH (PSI)	YOUNG'S MODULUS (PSI)	REMARKS
RS-1	B1-B	15+24	6' RT	-L-	51.8-52.5'	GRANITE	PPmg	100%	78%	5.0	1.96	168.8	5,460	-	-
RS-2	B2-A	15+70	9' LT	-L-	37.2-37.8'	GRANITE	PPmg	100%	45%	5.0	1.98	161.7	4,840	-	-